

# Large Scale Model Experiments of Recycled Pavement Materials

Professors Tuncer Edil, Jim Tinjum, Craig Benson, Students Brian Kootstra and Ali Ebrahimi

University of Wisconsin – Madison, Geo-Engineering Program



## Problem Statement

- Roads require continual maintenance and reconstruction, which can be costly and negatively impact the environment.
- Reconstruction with full-depth reclamation (FDR) utilizes existing roadway materials, saving time and money while avoiding landfill disposal and the use of virgin materials.
- Cement or fly ash is often blended with the old roadway materials to improve strength and stiffness.



Fig 1: Road reconstruction with FDR

## Resilient Modulus ( $M_r$ )

- $M_r$  is a key material property for roadway design, where  $\sigma_d$  is the deviator stress and  $\epsilon_r$  is the recoverable strain under repeated loading.
- $M_r$  of granular materials varies with the state of stress, and can be described with the relationship:

$$M_r = \frac{\sigma_d}{\epsilon_r}$$

where  $\sigma_b$  is the bulk stress, or sum of the 3 principal stresses,  $p_0$  is a reference stress, and  $k_1$  and  $k_2$  are empirical constants.

$$M_r = k_1 \left( \frac{\sigma_b}{p_0} \right)^{k_2}$$

## Large Scale Model Experiment (LSME)

- The LSME, shown in Fig. 2, measures loads and deflections, from which  $M_r$  is backcalculated.
- The LSME incorporates the effect of stress magnitude and strain amplitude on modulus, replicating field conditions more closely than conventional laboratory  $M_r$  testing.

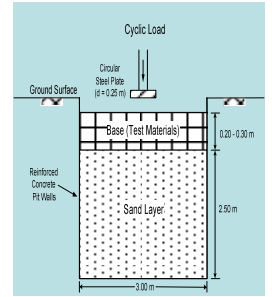
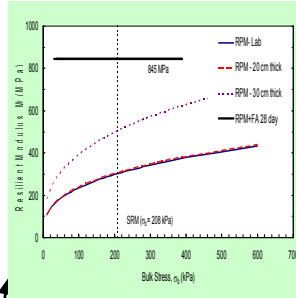


Fig 2: Schematic of LSME

## Results

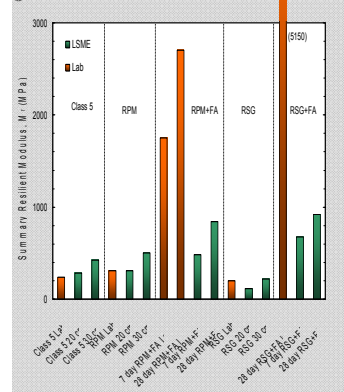
### Fig. 3: $M_r$ vs. Bulk Stress for RPM

- $M_r$  of unstabilized granular materials increases with bulk stress and layer thickness.
- Stabilizing the RPM, such as with fly ash (FA), results in a significant increase of  $M_r$ .
- Stabilized materials are not stress-dependent, as seen from the horizontal line in Fig. 3.



### Fig. 4: Comparison of $SM_r$

- The summary  $M_r$  ( $SM_r$ ) at  $\sigma_b = 208$  kPa are compared for base materials from the LSME and conventional lab testing.
- A significant increase occurs with fly ash, increasing over a curing time of 28 days.
- Lab tests for stabilized materials result in a higher  $SM_r$  than the LSME due to differences in mixing and curing methods.



## Tested Roadway Materials

- 3 granular base course materials were evaluated:
  - Class 5 gravel: a typical base course material
  - Recycled Pavement Material (RPM): 50% pulverized asphalt concrete and 50% granular base material
  - Road Surface Gravel (RSG): a gravel surface course

Material	$D_{50}$ (mm)	$C_u$	$C_c$	$G_s$	$W_{pi}$ (%)	$Y_{max}$ (kN/m)	Asphalt Content (%)	IL (%)	PL (%)	Grad. Cont. (%)	Str. Cont. (%)	Fire Cont. (%)	UCS (kPa)	ASHI Cont. (%)
Class 5 base	225	333	07	272	50	209	-	NP	NP	366	53	41	SP	A1a
RPM	389	895	25	264	75	212	46	NP	NP	460	48	106	GWC	A1a
RSG	080	400	10	273	75	226	-	21	14	286	50	124	SGSM	A24

## Conclusions and Applications

### Fig. 5: Gravel Equivalency

- Stabilized recycled materials offer a promising alternative due to increase in  $M_r$ .
- Recycled materials can be incorporated into base layer thickness design:
  - The AASHTO base layer coefficient ( $a_2$ ) can be calculated from  $M_r$  (Rada and Witczak 1981)
  - The thickness of an alternative recycled material can be determined from the required thickness of Class 5 gravel using Fig. 5.
- A thinner layer of recycled material stabilized with fly ash can be used as an alternative to Class 5 gravel.

$$a_2 = 0.240 M^{-0.97}$$

